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Daily reservoir inflow forecasting combining QPF into ANNs model

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Abstract

Daily reservoir inflow predictions with lead-times of several days are essential to the operational planning and scheduling of hydroelectric power system. The demand for quantitative precipitation forecasting (QPF) is increasing in hydropower operation with the dramatic advances in the numerical weather prediction (NWP) models. This paper presents a simple and an effective algorithm for daily reservoir inflow predictions which solicits the observed precipitation, forecasted precipitation from QPF as predictors and discharges in following 1 to 6 days as predicted targets for multilayer perceptron artificial neural networks (MLP-ANNs) modeling. An improved error back-propagation algorithm with self-adaptive learning rate and self-adaptive momentum coefficient is used to make the supervised training procedure more efficient in both time saving and search optimization. Several commonly used error measures are employed to evaluate the performance of the proposed model and the results, compared with that of ARIMA model, show that the proposed model is capable of obtaining satisfactory forecasting not only in goodness of fit but also in generalization. Furthermore, the presented algorithm is integrated into a practical software system which has been severed for daily inflow predictions with lead-times varying from 1 to 6 days of more than twenty reservoirs operated by the Fujian Province Grid Company, China.

1 Introduction

Daily reservoir inflow forecasting with lead-times of several days has important significance to the operational planning and scheduling of hydroelectric power system, such as minimizing the thermal power generation or peak-energy maximization. However, due to the complexity of hydrological system characterized by dynamic, uncertain, and nonlinear, forecasting of daily reservoir inflow is still one of the most difficult issues in hydrological science up to now. During the past few decades, a great deal of research has been devoted to the formulation and development of approaches and models to

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improve the quality of hydrological prediction, including mechanistic models (Jacquin and Shamseldin, 2006; Mwakalila et al., 2001; Refsgaard, 1997; Romanowicz, 2007; Yu-Chi et al., 2006) and black-box models (Jain and Srinivasulu, 2004; Lin et al., 2006; Liong and Sivapragasam, 2002; Nayak, 2008; Yu et al., 2006). The mechanistic models, which have been studied for more than fifty years and widely used nowadays, lay emphasis on the description of physical laws and try to make a comprehensive and incisive understanding of every step in rainfall-runoff (RR) processes. However, the mechanistic models used to model such processes would require a large amount of high-quality data associated with hydrological, meteorological, natural geographical characteristics as well as human activities, while the burden of data constrains the application of mechanistic models. For a large scale of hydropower system, selecting one of mechanistic models will be difficult because of lack of data. In contrast, the black-box models, that at first were only designed to identify the connection between inputs and outputs, are widely applied to forecast the streamflow because of their requirement of little data and their simple formulation. The earlier methods include time series techniques and multiple linear regression methods (Irvine and Eberhardt, 1992; James, 1991). As an alternative to the aforementioned mathematical models, ANNs, which map the input to output without the need to identify the physics a priori, have been widely applied to hydrology field (ASCE Task Committee, 2000; Atiya et al., 1999; Dawson and Wilby, 2001; Luk et al., 2000; Maier and Dandy, 2000). Some applications of ANNs in hydrological predictions can be found in literatures (Aqil et al., 2007; Birikundavyi et al., 2002; Campolo et al., 2003; Chau, 2006, 2007; Coulibaly et al., 2000; Dawson et al., 2006; Dibike and Solomatine, 2001; El-Din and Smith, 2002; Firat, 2008; Hu et al., 2005; Jain and Kumar, 2007; Karunasinghe and Liong, 2006; Kisi, 2004, 2007, 2008; Liong et al., 2000; Rajurkar et al., 2004; Shamseldin et al., 2002; Shamseldin et al., 2007; Wang et al., 2006; Wu and Chau, 2006).

With the rapid progresses of the Numerical Weather Prediction (NWP), quantitative precipitation forecasting (QPF) has been developed to an operationally feasible extent for hydrological forecasting (Collier and Kzyzstofowicz, 2000; Krzysztofow-

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icz and Collier, 2004) and gradually applied in some regions of the world reported by literatures in recent years. A European-scale flood forecasting system (EFFS) had been developed to make a probabilistic assessment of n -day ahead discharge exceedence risk ($n < 10$) with the help of the QPF from the European Centre for Medium Range Weather Forecasting (ECMWF) Ensemble Prediction System (De Roo et al., 2003). Brath et al. (2002) employed short-term rainfall forecasting as input of several non-parametric hydrological models to update the discharges forecasting and the results showed a remarkable improvement could be achieved compared with the discharges from conceptual model alone. Habets et al. (2004) used QPF from two numerical weather prediction models, namely ARPEGE and ALADIN, to forecast daily streamflow in Rhone basin, France, and concluded that the utility of precipitation prediction could improve the accuracy of long-duration floods forecasting. Collischonn et al. (2005) made use of QPF given by a regional numerical weather prediction model through a distributed hydrological model to perform a continuous 167-days discharge forecasting of River Uruguay basin, Brazil, and suggested that the usefulness of rainfall forecasting should be improved. His another work (Collischonn et al., 2007) demonstrated that the forecast errors could be reduced considerably comparing with ARMA model. Reggiani and Weerts (2008) outlined a methodology to produce probabilistic quantitative precipitation forecasting for river flow prediction.

China is one of the biggest developing countries in the world and hydropower power plays an increasingly important role in its modernization process. Because of energy shortage, environmental pollution, ecological destruction, etc., hydropower is strongly advocated by the recent National Five Year Plans and more reservoirs and hydropower plants will be built for power generation in next 10 years. Therefore, accurate daily reservoir inflow forecasting is urgently required for the operational planning and scheduling of hydroelectric power system. The new generation national Medium-Range Numerical Weather Prediction (NWP) system, named T213L31, was built in 2002 and the QPF information of the most regions in the country could be obtained from this system but there were few reports about its application in hydrological fore-

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casting. So, this study will make use of QPF information for daily reservoir inflow forecasting using a simple model – BP neural network. In order to make the training process more efficient, the standard back-propagation algorithm is improved by using self-adaptive learning rate and self-adaptive momentum coefficient which can import the error function variation into the adjustment of weights matrix and biases matrix. A statistic method which makes use of auto-correlation function (ACF) and cross-correlation function (CCF) functions is employed to specify the most efficient predictors' lags which were generally selected by empiricism. The proposed model is applied to daily reservoir inflow forecasting and the results are compared with a traditional linear model – Autoregressive Integrated Moving Average (ARIMA) model as the bench-mark model. Furthermore, the presented study is integrated into a practical software system which has been severd for daily inflow predictions with lead-times varying from 1 to 6 days of more than twenty reservoirs operated by the Fujian Province Grid Company, China. The context of this paper is organized as follows: The description of the study area and data collection is introduced in Sect. 2. Then, Sect. 3 discusses the methodology and modelling procedure of R-R simulation and the results are discussed in Sect. 4. The web-based daily reservoir inflow forecasting system (WDRIFS) which integrated the method, modeling and algorithm in this study is described in Sect. 5 and the conclusions are given in Sect. 6.

2 Study area and data collection

The study area is located in Fujian province, which owns the most important regional power grids of China and hydropower makes approximate 50% contribution of the whole electric power generation capacity. This area, as shown in Fig. 1, is bounded by the Taiwan Strait and Zhejiang, Jiangxi, and Guangdong provinces, southeastern China between $23^{\circ}31' - 28^{\circ}19' N$ and $115^{\circ}50' - 120^{\circ}44' E$, and is comprised of eight hydrographic basins named Minjiang, Jiulongjiang, Jinjiang, Tingjiang, Jiaoxi, Ao'jiang, Huotongxi, and Mulanxi. Because of the typical sub-tropical oceanic monsoon climate

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and mountain landforms, this area suffers from abundant but uneven temporal and spatial distributed precipitation with the mean annual rainfall between 1400 mm and 2000 mm and more than 70% of that occurs in summer from April to September. The runoff in this area has similar characteristics as the precipitation makes the major contribution to its generation. The mean annual runoff volume of Minjiang which is the largest river in this area and has a drainage area of 61 000 km², is approximate 63 billion cubic meters. This numeral occupies 7th place in China and exceeds about 10% that of the Yellow River whose drainage area is 12 times bigger than that of Minjiang.

There are about sixty reservoirs in this area and more than twenty of them, which locate in main stream and have regulating storage, are selected in this study due to their great significance to the downriver industry and population. Although discharge and rainfall have been recorded since 1990 for most of the reservoirs, some of the data in initial years were not verified or lost, in addition, the condition of some basins had been changed by the implementation of new hydrological projects during 1990–2000, so, daily average discharges and rainfalls from 1 January 2000 to 31 December 2007 are used to calibrate and validate the model. The QPF information is collected from nine regions forecasted for the same day and next two days by the new generational medium-range NWP system in national meteorological observatory of China at 08:00 a.m. every day.

Shuikou, which is the most important reservoir in the study area, will be taken as an example and illustrated in detail for the modelling procedure and result analysis.

3 Methodology and modeling

3.1 Predictors and forecasting targets

There are three variables which have relation to the reservoir daily inflow as follows: the antecedent observation of rainfall and discharge, the QPF for following days and the seasonal information.

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For every day t , four MLP-ANNs models, namely Model($t+i$) $i=0, 1, 2, 3$, are developed for the daily reservoir inflow forecasting with lead-times (represented by symbol i) varying from 1 to 6 days. The Model($t+i$) $i=0, 1, 2$ makes use of the different QPF for the first three days whereas the Model($t+3$) is a unified model for the next four days since no QPF more than three days is available at present.

A statistic perspective, proposed by Sudheer (2002), is used here to select the antecedent predictors which have the most significant effect on the inflow of following days. By means of this approach, the lag number of antecedent discharge is determined by auto-correlation function (ACF) and the lag number of antecedent rainfall is decided by cross-correlation function (CCF). For Shuikou reservoir, the results of ACF and CCF statistic analysis, as shown in Fig. 2, indicate that antecedent one-day discharge (max ACF=0.857) and antecedent two-day rainfalls (max CCF=0.678) should be chosen into the predictors.

So, the models can be demonstrated by four equations as follows:

Model($t+0$):

$$Q(t+1) = f(P(t-2), P(t-1), Q(t-1), QPF(t), \bar{P}(30), \bar{Q}(30)) \quad (1)$$

Model($t+1$):

$$Q(t+1) = f(P(t-2), P(t-1), Q(t-1), QPF(t), QPF(t+1), \bar{P}(30), \bar{Q}(30)) \quad (2)$$

Model($t+2$):

$$Q(t+2) = f(P(t-2), P(t-1), Q(t-1), QPF(t), QPF(t+1), QPF(t+2), \bar{P}(30), \bar{Q}(30)) \quad (3)$$

Model($t+3$):

$$Q(t+i) = \text{Model}(t+0) \quad i = 3, 4, 5, 6; \quad QPF(t) = 0 \quad (4)$$

in which QPF is the quantitative precipitation forecasting, P and Q are the antecedent observed rainfall and discharge, $\bar{P}(n)$ and $\bar{Q}(n)$ are the mean values of the observed rainfall and discharge of antecedent n days as the seasonal information.

3.2 Architecture of MLP-ANNs models

The MLP-ANNs used in this study is a feed forward neural network which generally consists of three layers viz. input, hidden, and output layer. There are a mass of interconnected and parallel processed neurons (namely node) in every layer, and each neuron can transfer an input of vector variable to an output of scalar variable through a nonlinear function called “Sigmoid”. The input layer is connected with the predictors outside, synchronously the output layer connects to the predictands, and the hidden layer is the connective bridge of them. The signal from “normalized” predictors, according to “feed forward” mode, is transmitted in order from the input layer through the hidden layer to the output layer, and is finally converted to the predictands.

The determination of the number of neurons in each layer is also an essential task. For the input layer and the output layer, the neurons number can be easily specified according to the number of predictors and predictands, respectively. However, the determination of neuron number in hidden layer, which is crucial for the success of the networks, is still a difficult procedure until now. So, an experiment with a trial-and-error measure, recommended as the best strategy by Shamseldin (1997), is used in our work. The experimental results indicate that the numbers of neurons in the hidden layer, for Shuikou reservoir, are 12, 15 and 20 for Model($t+0$), Model($t+1$) and Model($t+2$), respectively.

3.3 Improved error back-propagation algorithm

The error back-propagation algorithm is widely used to adjust the weights matrix and the biases matrix of the networks through an iterative training procedure for the purpose of revealing the relationship between the predictors and the predictands. From an initial random weights matrix and bias matrix, this algorithm searches the “weight space” by using gradient descent method to minimize the overall error between the simulated outputs and the observed values.

A method with self-adaptive learning rate and self-adaptive momentum coefficient is

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used here to accelerate the training process and to prevent the algorithm from converging at a local minimum. The learning rate α and the momentum coefficient β are automatically adjusted as follows (Sun et al., 2007):

(a) when $e_r^{(k)} < 0$

$$\begin{cases} \alpha(k+1) = \alpha(k)[1 + u \cdot e^{-e_r^{(k)}}] \\ \beta(k+1) = \beta(k)[1 + v \cdot e^{-e_r^{(k)}}] \end{cases} \quad (5)$$

(b) when $e_r^{(k)} \geq 0$

$$\begin{cases} \alpha(k+1) = \alpha(k)[1 - u \cdot e^{-e_r^{(k)}}] \\ \beta(k+1) = \beta(k)[1 - v \cdot e^{-e_r^{(k)}}] \end{cases} \quad (6)$$

where k is the epoch index, $u, v \in (0, 1)$ are two parameters (0.5 and 0.5 in this study), and $e_r^{(k)}$ is an artificial coefficient, indicates the direction of the overall error which is defined as follows:

$$e_r^{(k)} = \frac{\Delta E(k)}{E(k)} = \frac{E(k) - E(k-1)}{E(k)} \quad (7)$$

By this dynamical adjustment, the learning rate increases when the overall error reduces and decreases when the overall error increases over a certain rate. Additional momentum term is used to import the effect caused by the changes of the weights matrix in the previous epoch and this item can effectively smooth the descending path and prohibit converging at a local minimum. So, the change of weights matrix w_{ji} and bias matrix q_j are adjusted as follows:

$$\begin{cases} \Delta w_{ji}(k+1) = \sum_{p=1}^P \alpha_j d_{pj} x_i + \beta_j \Delta w_{ji}(k) \\ \Delta q_j(k+1) = \sum_{p=1}^P \alpha_j d_{pj} + \beta_j \Delta q_j(k) \end{cases} \quad (8)$$

in which $p=1, 2, \dots, P$ presents the p th training pattern.

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3.4 Performance assessment measures

A number of error measures (Dawson et al., 2007; Legates and McCabe, 1999) have been developed to assess the goodness of fit performance of hydrological forecasting models but no standard has been specified since each measure can just assess one or two aspects of the runoff characteristics. Five commonly used error measures, therefore, are employed in this study to make the evaluation of the forecasting. They are the coefficient of efficiency (CE), the mean absolute error (MAE), the mean squared relative error (MSRE), the relative volume error (RVE) and the coefficient of determination (R^2), respectively defined as follows:

$$CE = 1 - \frac{\sum_{i=1}^n (Q_i - \hat{Q}_i)^2}{\sum_{i=1}^n (Q_i - \bar{Q})^2} \quad (9)$$

$$MAE = \frac{\sum_{i=1}^n |Q_i - \hat{Q}_i|}{n} \quad (10)$$

$$MSRE = \frac{\sum_{i=1}^n \frac{(Q_i - \hat{Q}_i)^2}{Q_i^2}}{n} \quad (11)$$

$$RVE = \frac{\sum_{i=1}^n (Q_i - \hat{Q}_i)}{\sum_{i=1}^n (Q_i)} \quad (12)$$

$$R^2 = \left[\frac{\sum_{i=1}^n (Q_i - \bar{Q})(\hat{Q}_i - \tilde{Q})}{\sqrt{\sum_{i=1}^n (Q_i - \bar{Q})^2 (\hat{Q}_i - \tilde{Q})^2}} \right]^2 \quad (13)$$

where Q_i is the observed discharge, \hat{Q}_i is the simulated discharge, \bar{Q} is the mean of the observed discharges, \tilde{Q} is the mean of the simulated discharges and n is the length of the observed/simulated series.

The CE (Nash and Sutcliffe, 1970), which ranges from $-\infty$ to 1, describes the rela-

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5 tionship between the modeled value and the mean of the observed data. A coefficient of one ($CE=1$) means that the model performs a perfect matching to the observed data, a coefficient of zero ($CE=0$) indicates that the modeled result just has the equal mean value with the observed data, whereas a coefficient of negative value ($CE<0$) shows that the model performs worse than using the observed mean.

The MAE, which ranges from 0 to $+\infty$, is used to measure how close forecasts are to the eventual outcomes. Theoretically, a coefficient of zero ($MAE=0$) means the best model with a perfect performance.

10 The MSRE, which ranges from 0 to $+\infty$, can provide a balanced evaluation of the goodness of fit of the model as it is more sensitive to the larger relative errors caused by the low valued and the best coefficient will be zero ($MSRE=0$).

The RVE, gives the relative bias of the overall water balance of the model and the best coefficient will be zero ($RVE=0$).

15 The R^2 , which ranges from 0 to 1, is a statistical measure of how well the regression line close to the observed data and a coefficient of one ($R^2=1$) indicates that the regression line perfectly fits the observed data.

4 Results and discussions

20 The goodness of fit performances of calibration from 1 January 2000 to 31 December 2005 and validation from 1 January 2006 to 31 December 2007 are illustrated in Figs. 3 and 4, respectively, and the statistic result of error measures is given in Table 1. Figure 5 shows the relevant scatter diagrams of simulated discharge versus observed discharge of $Model(t+0)$, $Model(t+1)$, $Model(t+2)$ and ARIMA in calibration (Fig. 5a–d) and validation (Fig. 5e–h).

25 Figures 3 and 4 present the daily rainfall sequence and discharge sequence continually and the simulated results from three different models are synchronously given with the observed records in each figure. The simulated curves in both Figs. 3 and 4 clearly indicate that not only the rising trends and the falling trends in the hydrograph

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are picked up by Model($t+0$), Model($t+1$) and Model($t+2$) but also excellent goodness of fit performances are achieved.

As shown in Table 1, the general quantitative scope of the five error measures indicates that the before-mentioned models are suitable for the RR process simulation.

Satisfactory forecasting is obtained in this study since the CE and R^2 are sufficiently high close to 1, and the MSRE and RVE are adequately low approximates 0. The measures MAE of calibration and validation are far less than the relevant mean value of the observed data, 1494.46 m³/s and 1643.25 m³/s, respectively. The high scores of CE and R^2 indicates that all the models present the “Best” performance according to the standard given by Dawson et al. (2007).

From the scatter diagrams in Fig. 5, it is obviously that both of the low values and the high values are close to the exact fit line and this result suggests that there is no evident overestimate or underestimate occurs during the simulation.

The statistic result of error measures of the validation are as much as that of the calibration and both of them are encouraging. This outcome implies that the training procedures are successful without “overtraining” or “local minimum” and the proposed models have powerful generalization abilities for out-of-sample forecasting.

The performance of the proposed model is also evaluated by comparing the forecasting results with that of a traditional linear model – ARIMA (p, d, q) model and the basic parameters of ARIMA model – p, d , and q are specified to be 4, 1 and 2, respectively, in this paper. As listed in Table 1, four error measures CE, MAE, MSRE and R^2 show that all the MLP-ANNs models perform much better than ARIMA model and only the RVE indicates that the ARIMA model is better. Furthermore, the scatter diagrams, as shown in Fig. 5, show that the forecasted results of our models, compared with that of ARIMA model, are much more close to the exact fit line, especially for the bigger inflows. Therefore, it is clearly that the presented model outperforms ARIMA model according to our simulating experiment.

In order to investigate the affection of QPF on forecasting, we recalculated the calibration data set and the validation data set and the QPF information is eliminated from

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the input vector this time. From the quantitative results of the error measures which are listed in Table 2, it is clearly that the scores of all error measures are decreased compared with that in Table 1. Further more, with the lead-times increasing, the performances deteriorate more obviously (The scores decrease less than 5% for Model($t+0$), 5%–8% for Model($t+1$), and 8%–15% for Model($t+2$)).

5 Software implementation

On the basis of the previous study, an integrated web-based daily reservoir inflow forecasting system (WDRIFS), implemented within java technology framework, has been successfully developed recently and been applied in daily inflow predictions of more than twenty reservoirs operated by the Fujian Province Grid Company, China, for the purpose of making operational planning and scheduling of hydroelectric power system.

The WDRIFS is the newest version production of hydrological forecasting software after the Chinese national flood control system for reservoirs (Cheng and Chau, 2004) and the Web-based flood forecasting system (Cheng et al., 2004). Comparing with the two existed system, the WDRIFS puts more focus on the daily forecasting with relative long lead-times and the QPF information is firstly applied to the RR simulation in this system in China. The WDRIFS is developed under the condition of JBuilder 2006, Weblogic 8i and Oracle 10g, conformed to the J2EE criterion and a number of java techniques, such as Applet, Servlet, Swing, JDBC, JSP and so on, are used during the implementation. By the aid of this system, a web-based forecasting of daily reservoir inflow with lead-times varying from 1 to 6 days will be automatically accomplished in time every day after the QPF is released.

This system is composed of three main function modules, namely parameter calibration module, auto forecasting module and simulation forecasting module, respectively. Parameter calibration module is used to determine the relevant parameters of the MLP-ANNs models for every reservoir. For each model, some items should be specified before training the networks as follows: the historical data sets for calibra-

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tion and validation, the lag number of antecedent predictors, the number of neurons in hidden layer and the termination condition. While the training procedure is processing, as shown in Fig. 6, the simulated discharge is painted together with the observed discharge dynamically and the hydrograph can be zoomed in/zoomed out by dragging with mouse right button pressed to inspect the local part of the discharge sequence such as the flood peak. At the same time, the numerical simulated result and the quantitative statistic result of error measures are listed in two tables, respectively. Two error measures besides the aforementioned ones, namely mean absolute percentage error (MAPE) and qualified percentage (QP) which means the percent of the simulated discharge with a relative error less than the specified standard, are added in this system due to their popularly usage in China. The training procedure can be paused or resumed and the hydrograph can be switched over calibration and validation. The optimal parameters will be traced and recorded to the database after the training procedure is accomplished, ended by the specified max epoch, CE, qualified percentage or the combination of them. Figure 7 shows the auto forecasting interface in which the auto forecasting plans and the QPF information together with the observed data for following six days are listed in one table. Some statistic information such as the relevant error and the qualified percentage is displayed by default and it can be hidden by changing the table header setting. The graphics mode (see Fig. 8) will be displayed by clicking the table header, through which the goodness of fit performance of forecasting can be evaluated by comparing the simulated hydrograph with the observed hydrograph. When the QPF information is trustless or urgent hydropower scheduling tasks need to be arranged in advance in flood season, the seasoned hydrological experts or operators can design or assume the QPF information and make a simulation forecasting based on it (see Fig. 9). In this interface, the QPF chart (the smaller one) is painted besides the forecasting chart and the QPF for every day can be reassigned by directly input in the last column of the table, or by dragging with mouse left button pressed. Once the QPF is changed by any way, the simulated discharge will be recalculated immediately and the forecasting chart will update. The approbatory simulation

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forecasting result will be saved as a manual plan of that day.

6 Conclusions

The future precipitation is the dominant source of uncertainty in hydrological forecasting and the current scientific advances in the QPF are feasible (Krzysztofowicz and Collier, 2004). So, any research on integrating QPF into hydrological models is significant and promising for improving the quality of hydrological forecasting. This paper combines the QPF into ANNs models to perform daily reservoir inflow forecasting. Different models based on MLP-ANNs, in which the improved error back-propagation algorithm with self-adaptive learning rate and self-adaptive momentum coefficient is used to make the supervised training procedure more efficient both in time saving and search optimization, are developed to simulate daily reservoir inflow with lead-times of several days. The experimental results indicate that these models can extend the forecasting lead-times with a satisfactory goodness of fit performance and the precision of the forecasting results are much better than traditional ARIMA model. As the application of this study, an integrated web-based daily reservoir inflow forecasting system (WDRIFS), implemented within java technology framework, is developed and has been severd for daily inflow predictions with lead-times varying from 1 to 6 days of more than twenty reservoirs operated by the Fujian Province Grid Company, China. The primary interfaces and functions of this system are presented in detail in this paper.

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Table 1. The statistic result of error measures of calibration and validation.

Error measures	Calibration				Validation			
	Model ($t+0$)	Model ($t+1$)	Model ($t+2$)	ARIMA	Model ($t+0$)	Model ($t+1$)	Model ($t+2$)	ARIMA
CE	0.9687	0.9682	0.9661	0.7486	0.9520	0.9263	0.9499	0.7670
MAE	200.14	201.51	212.72	384.03	240.28	255.49	255.20	443.46
MSRE	0.0554	0.0678	0.0552	0.1273	0.0450	0.0572	0.0649	0.1525
RVE	0.0433	0.0306	0.0785	0.0013	0.0498	0.0536	0.0682	0.0030
R^2	0.9710	0.9688	0.9719	0.7476	0.9534	0.9360	0.9565	0.7675

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Table 2. The statistic result of error measures of calibration and validation (without *QPF* information).

Error measures	Calibration			Validation		
	Model ($t+0$)	Model ($t+1$)	Model ($t+2$)	Model ($t+0$)	Model ($t+1$)	Model ($t+2$)
CE	0.9332	0.8924	0.8903	0.9173	0.8565	0.8067
MAE	208.35	216.50	222.62	251.12	271.14	276.24
MSRE	0.0561	0.0721	0.0623	0.0482	0.0610	0.0634
RVE	0.0450	0.0326	0.0841	0.0512	0.0564	0.0771
R^2	0.9322	0.9156	0.8796	0.9187	0.8704	0.8424

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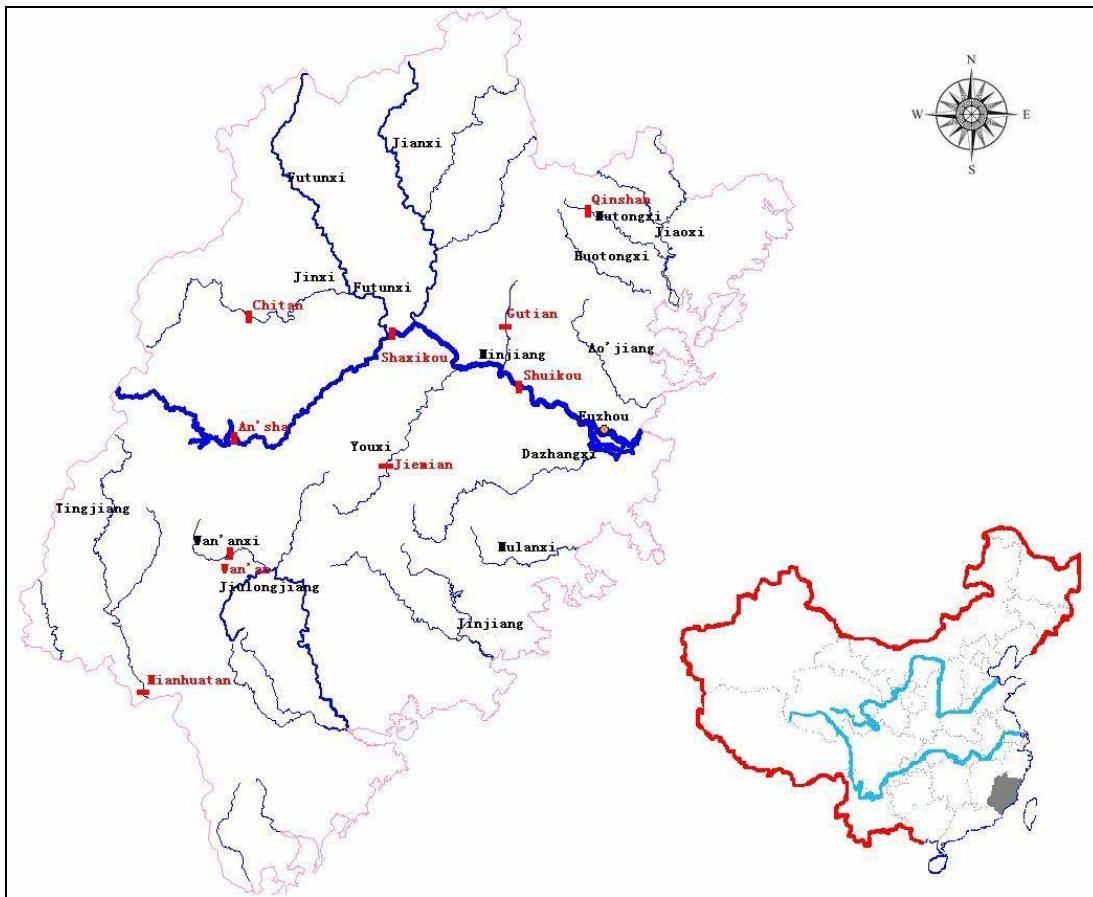


Fig. 1. The study area.

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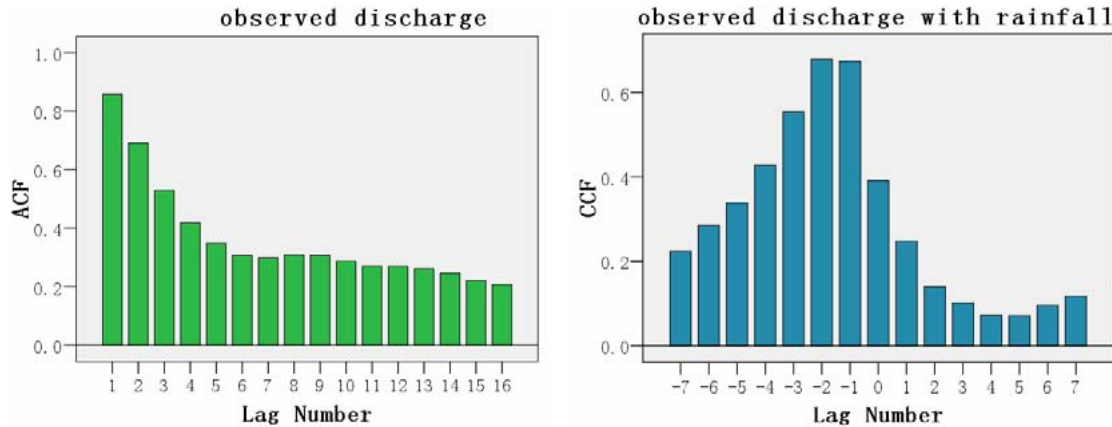


Fig. 2. The ACF and CCF statistic analysis of Shuikou reservoir.

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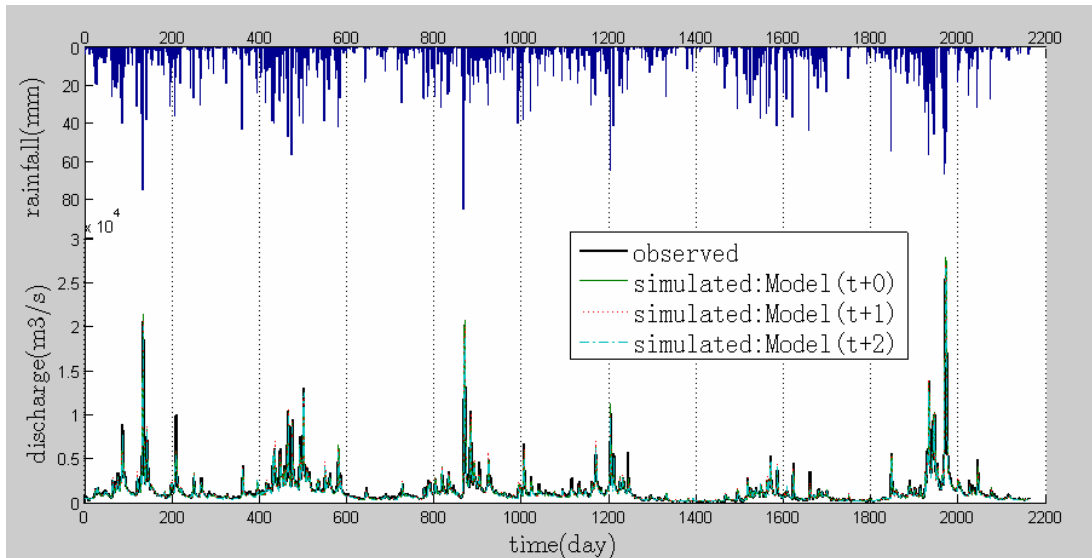


Fig. 3. Calibration result of Shuikou reservoir (2000–2005).

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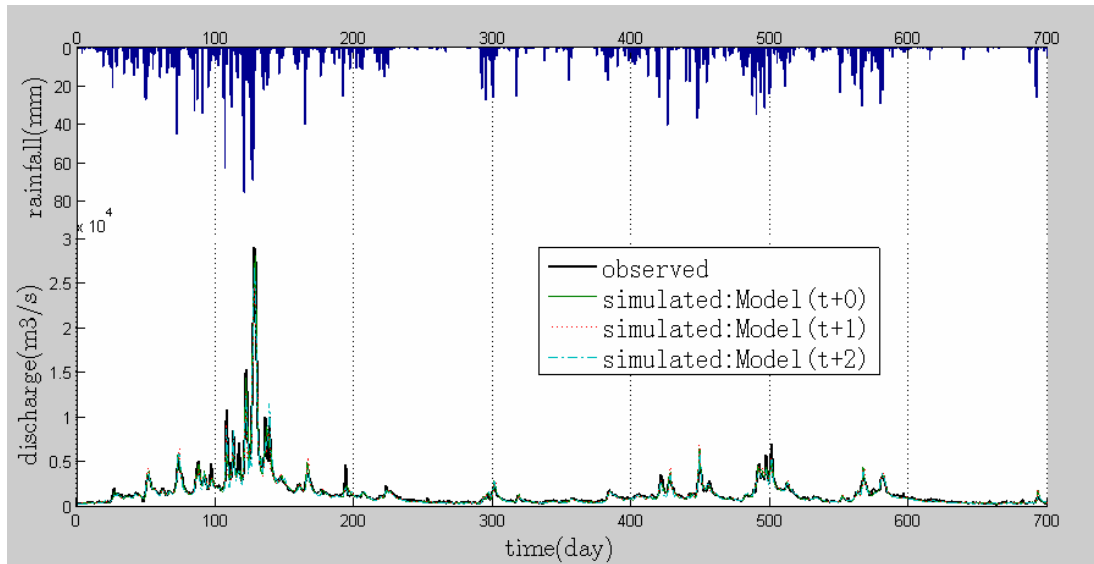


Fig. 4. Validation result of Shuikou reservoir (2006–2007).

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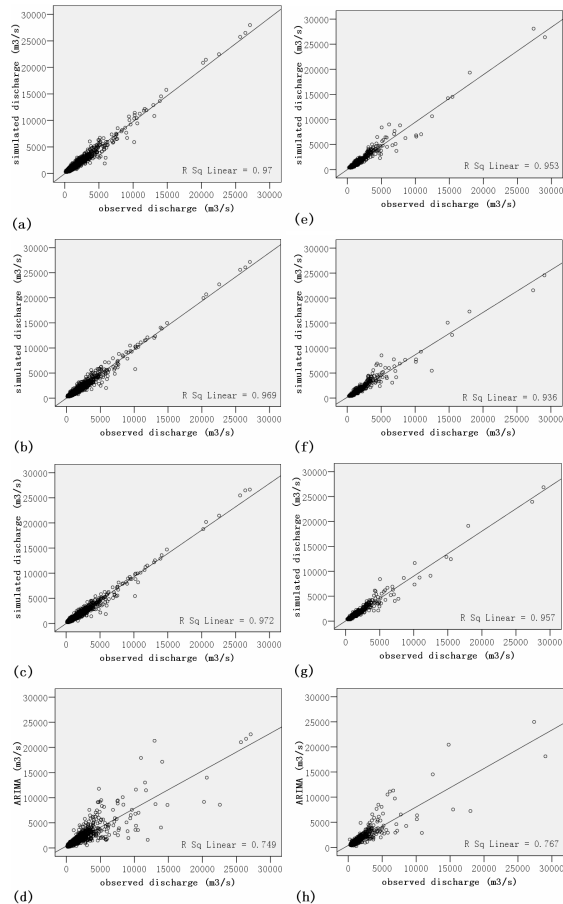


Fig. 5. Scatter diagrams of observed discharge and simulated discharge, **(a)** Model($t+0$) in calibration, **(b)** Model($t+1$) in calibration, **(c)** Model($t+2$) in calibration, **(d)** ARIMA in calibration, **(e)** Model($t+0$) in validation, **(f)** Model($t+1$) in validation, **(g)** Model($t+2$) in validation, **(h)** ARIMA in validation.

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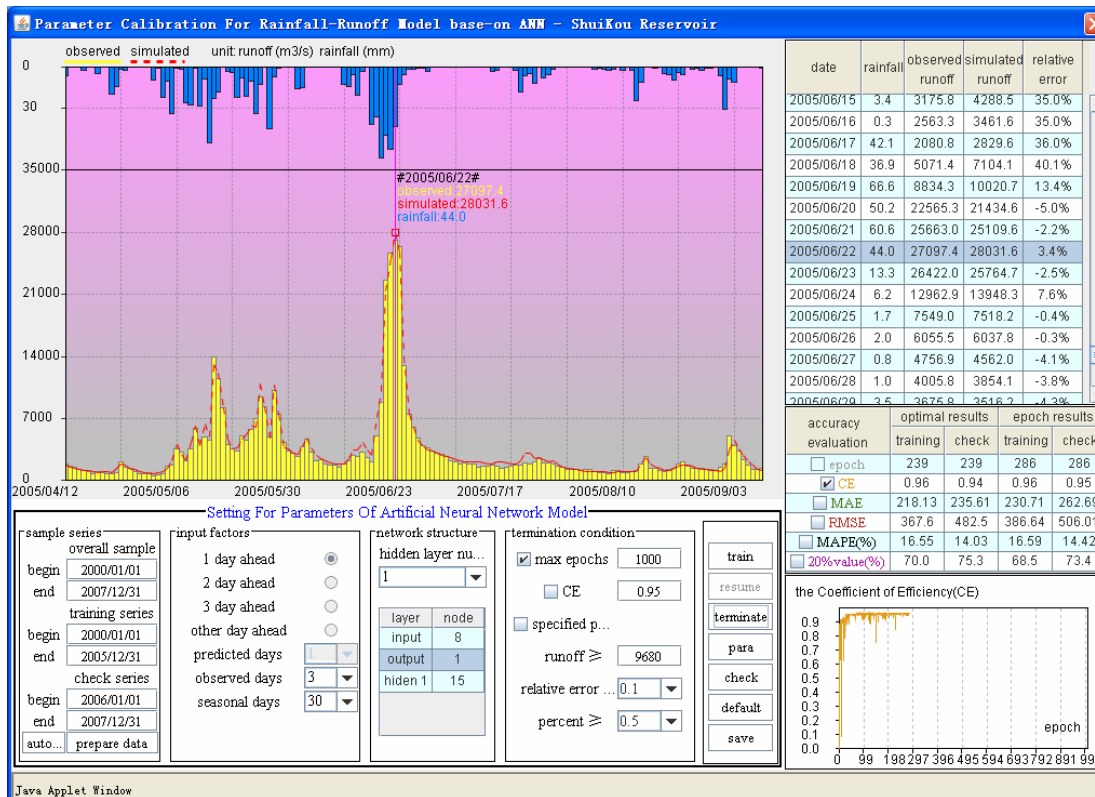


Fig. 6. The parameter calibration module.

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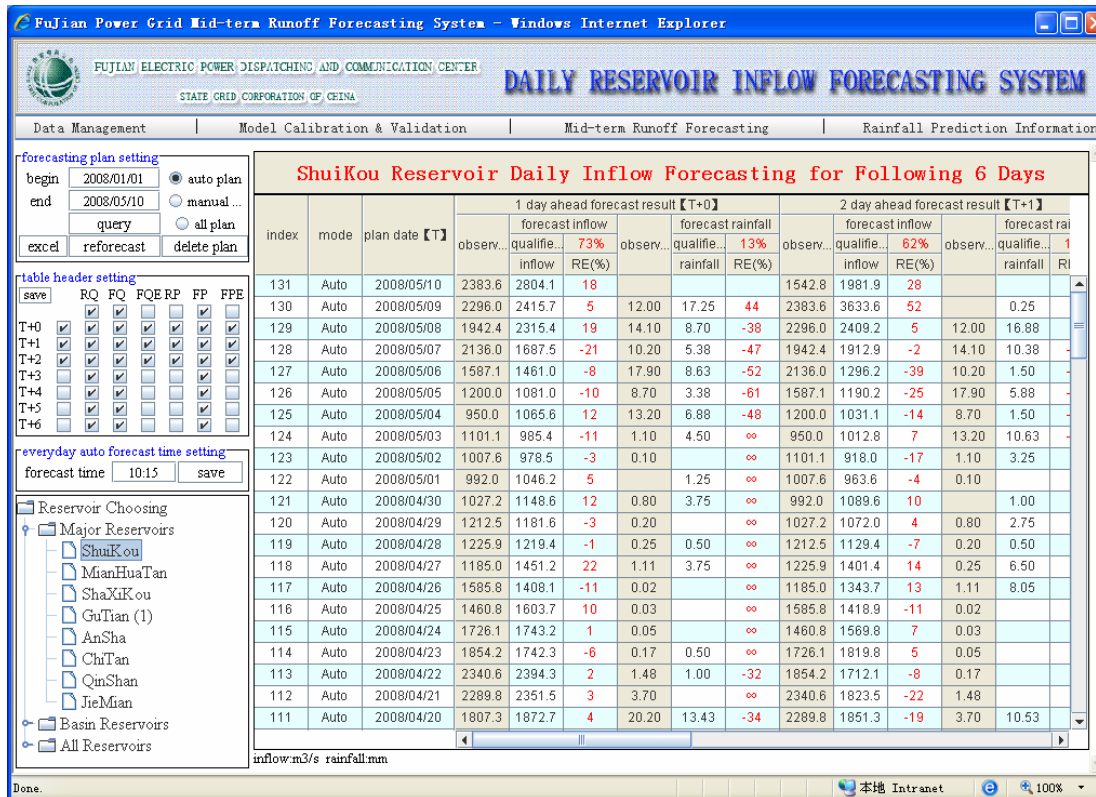


Fig. 7. The auto forecasting module.

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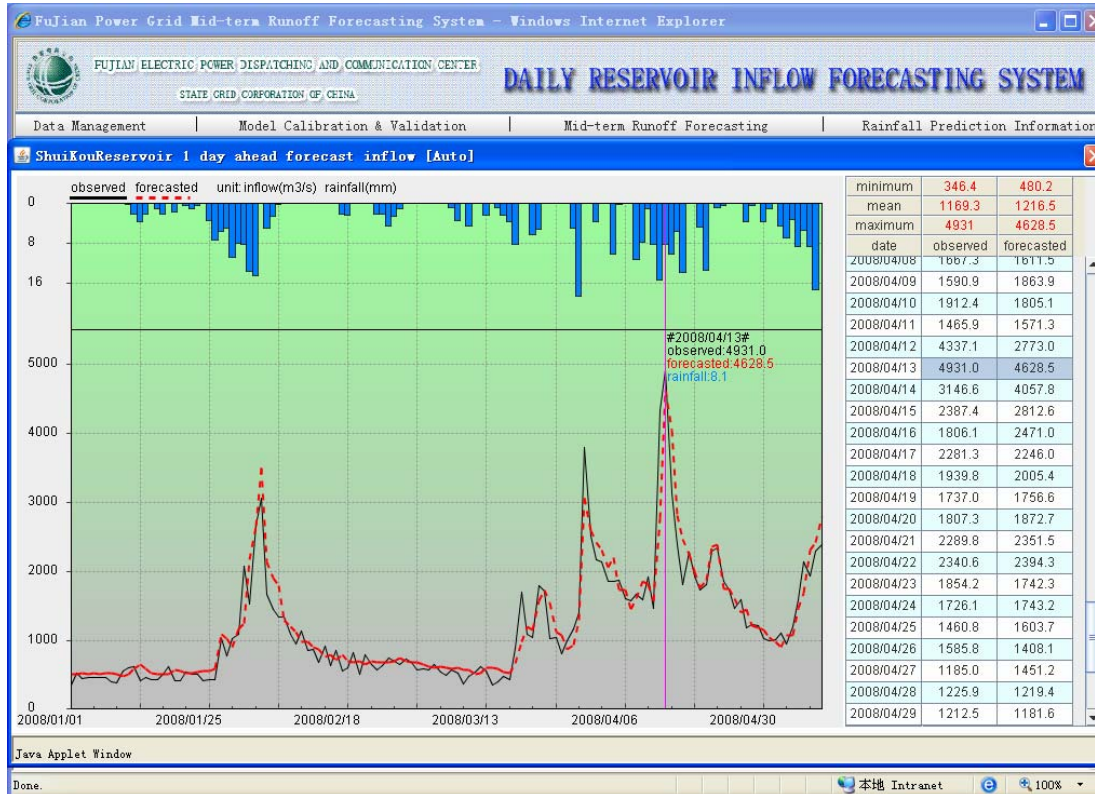


Fig. 8. The forecasted hydrograph and the observed hydrograph (Model($t+0$)).

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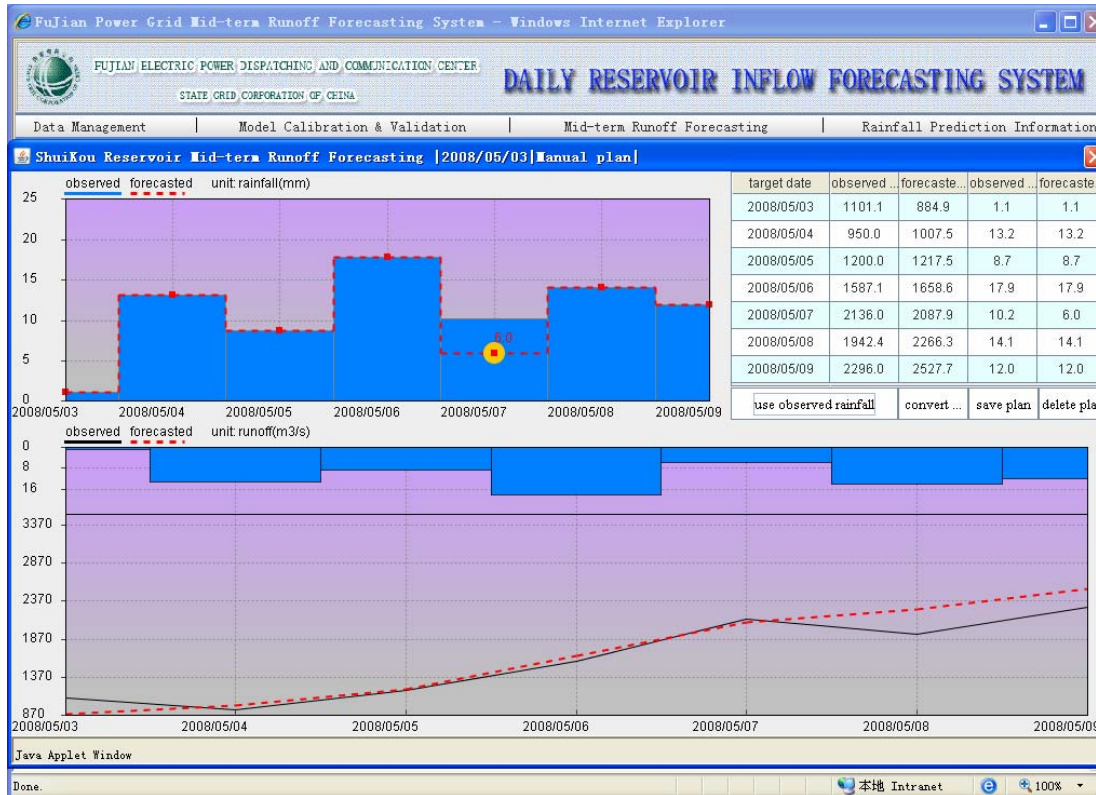


Fig. 9. The simulation forecasting module.

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