DEPARTEMENT DES ETUDES MECANIQUES ET THERMIQUES

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Conference on pipework design and operation London (UK) 19-20 Feb 1985 CEA-CONF—7834

- **MILLARD, D; HOFFMANN, A.**
- **PIPE WHIP ANALYSIS USING THE TEDEL CODE**

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THE LUI DOOD POHASSE PROGRAMMENT DEL ACTECHNIK.
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1. INTRODUCTION

in this opium In view of their abundance, piping systems are one of the main components in power industries and in particular in nuclear power plants. They must be designed for normal as well as faulted conditions, for safety requirements. For
example, in case of a sudden break, the pipe whip must be studied in order to determine if the free pipe may damage neighbouring structures like other pipes, concrete containments, etc...
The prediction of the dynamic behaviour of the free pipe requires accounting for several nonlinearities. For this purpose, a beam type fini-
te element program (TEDEL) has been used. The aim of this paper is to enlight the main features of this program, when applied to pipe whip analysis. An example of application to a real
case will also be presented. note

2. MAIN FEATURES OF PIPE MHIP ANALYSIS

Piping systems are rather flexible structures. This is mainly due to the presence of pipe
bends, where the cross-section tends to ovalize. This deformation is accompanied by higher stresses across the thickness of the pipe. In case of an instantaneous circumferential guillotine break, the pipe becomes free to move and the fluid which was initially at a high pressure will communicate its energy to the pipe. This is most of the time depicted by the introduction of
a following jet force, applied at the free end
of the pipe. As a consequence, the piping system or the pipe. As a consequence, the piping system
will undergo very large displacements, which
will be amplified by the developments of plas-
tic hinges along the pipe, due to the high level
of the stresses, in particular i counted for in a pipe whip analysis are the following:

- Geometric non-linearity, due to the large \pm is placements. Indeed it handles of large \pm translations and rotations, but strains can be
supposed to remain small entity germannable accuracy. 12 Charakter per 25.4 mm).
- Material non-linearity, since the stresses

will overstop the yield stress. The spread of the plastic zones across a given cross-section will result in a plastic hinge, in the usual sense of

result in a plastic hinge, in the usual sense of
strength of materials. dadic
force is contributed remains tangent
to the pipe. Such a long is force, which remains tangent
to the pipe. Such a long is non-conservative and
 stiffness at the contact, to account for local crusch rigidities.

A typical configuration for the pipe whip
analysis is shown on Figure 1.

analysis is shown on rigure i.
It is important to note that all these.
features must be modelled in the analysis since
they have a strong influence on the impact forces
which are the desired results.

From a theoretical point of view, the com-
plete problem is much complex since the only data

From a theoretical complex since the only data
are the initial conditions (state of stresses and
pressure in the pipe) and the duration of the
guillotine break at some given location.
Consequently the fluid and the struct

velocities and densities, or to use global infor-
mation, i.e. the jet .orce. The choice of one or the other solution may depend on the geometry of the piping system.

From a computational point of view, the use
of tridimensionnal shell finite elements would lead to a prohibitive cost. Consequently, it is necessary to use a beam-type finite element program, which will supply the global behaviour of the pipe.

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GENERAL PRESENTATION OF THE TEDEL PROGRAM $3₁$ TB<u>JT</u> 9 J

TEDEL is a general purpose finite element
program of the CEASENT System, Speciality of the
ted towards the calculation of tridimensionnal
structures mode or beams on the same of the calculation structures mode or beams or pipes. The main ele-
ments are that shell elements. Some special elements
are average for pipening analysis (see Pigure 2):

time curved elbow element,

- mitred elbow.

- T junction,

- tubes bundle element,

- contact element,

- coaxial tubes, with a sheet of water,
- elements with a sudden broadening or

shrinking of the cross-section, for fluid-structure interactions. Staffe modes may have six or eight degrees of

freedom, which are the three translations, the three rotations and for the fluid discretisation, the pressure **p** and an axiliary variable **x** which
obeys the following equation $x a \leq x \leq 1$.

$-p = \frac{d^2\pi}{dt^2}$

Following the classical concepts of usual strength of materials, the dual variables are ge neralized stresses, which are the classical axial
and shear forces, bending moements and torque.

For the material behaviour, calculations may be performed in elasticity, plasticity with isotropic or kinemactic strain-hardening, or creep with any law for the creep rate according to the user's choice :

provide in non column $\frac{d\epsilon^*}{dt} = f(\sigma^*, \ \epsilon^*, \ 1, \ t \dots)$

Where e* is an equivalent plastic strain, $\sigma^{\mathtt{a}}$ is an equivalent stress, and I the temperature. Besides, a special attention has been given to the modelisation of reinforced concrete behaviour laws : a model has been derived to account For the cracks in the concrete and the plastic
deformation of the reinforcements, as well as
for cyclic effects (unloading and reloading)
 $\sqrt{5}$, $\sqrt{6}$, $\sqrt{2}$, (see Figure 3).
A large variety of loads may be applied

the structure :

- nodal forces, rina siecrire zvad type B weight (including the weight of the fluid)

- inner pressure,

uniform rotation, none noire

and the control of the con-

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- distributed Toads,

acceleration et carameres at les

- thermal loads can marked these the fluid).
- local pressure variation (for the fluid). Concerning the boundary conditions, displacements can be prescribed at some nodes ; equalities as well as inequalities can be written between degrees of freedom. For fluid-structure interactions, some given pressure or flow can be
prescribed at a node.

Various types of analysis are possible using the JEDEL program :

Static analysis, either with a linear or a non linear material, and assuming small or large displacements. and Moreover, it is possible to calculate the
EULER's critical load in the frame of the usual elastic stability theory, or to de-
termine an elastoplastic critical load by using some displacement monitoring strategy to compute the response of the struc-
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. Dynamic analysis - either in the elastic linear domain : modal analysis ; response analysis by modal recombination ; seismic analysis by quadratic recombination of the
various modal responses, including a correction for neglected modes, and the pos-
sibility of different spectra at different nodes ; influence of an initial state of stresses on the frequencies ; account for viscous damping.

- or in the non linear (material and/or geometric) domain, by direct integration, using a Newmark type alcorithm.

An interesting feature of the TEDEL progra

An interesting reature of the HDEL program
is the possibility to calculate the fully coupled
acoustico-mechanical behaviour of the pipe.
For example, phenomena Tike the forced vibra-
tions of a pipe, induced by the flow, the transfert functions at one point of the cir-
cuit due to some unit signal at another point.

acoustico-mechanical response of the pipe to the various loads already mentioned, accounting for
non linear phenomena like the cavitation, the growth of a gaz bubble, the burst of a safety mbrane, the variation of sound celerity and fluid density, etc..., and for singularities due
to geometries changes (see the various elements already mentioned). Therefore, with such a pro-
gram, it is possible to treat in a fully coupled way, the very first milliseconds of the pipe whip as long as the acoustic effects are predominant in front of the convective terms for the fluid. Then, it is necessary to use a computer code like
PLEXUS. Many of the possibilities of the TEDEL
program have been validated by comparisons between calculations and either experimental or analytical solutions. Some of them are given in $ref. [12].$

4. SOLUTIONS ADOPTED FOR PIPE WHIP ANALYSIS

In order to treat the pipe whip problem at a reasonnable cost but with a sufficient accuracy, the following solutions have been adopted in
the TEDEL program for the main problems already mentionned in $§$ 2.

4.1 - Geometrical.non_linearity ("17_7

In case of large displacements, equilibrium must be written on the final configuration, which is a priori unknown. Starting from a configuration Ct where the Cauchy stresses of are
in equilibrium with loads F_t , one must find the
displacements and stresses increments due to a load increment AF such as :

 $F_{t+\Delta t}$ * F_t + ΔF

Equilibrium equation can be written as :

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equilibrium on C_{t+At} which is unknown.

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Where B is the matrix giving the strains in terms of the displacements.

This second equation is solved by means of an **updated kagrangian sechnique; texi worlds.** In brium equations on C₁+ At are convected on C₁;
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$$
here \, J_{t+\Delta t} = \frac{a_1 t + \Delta t}{d_1 t}
$$

If x denotes the 2nd Piola-Kirchoff stress tensor, we have :

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horeover, in the local axis of the element, we ressume that a _{bage} times

$$
\mathcal{I}_{t+\Delta t} \stackrel{\simeq}{\longrightarrow} \mathcal{I}_{t+\Delta t} = \mathcal{I}_{t} \stackrel{\simeq}{\longrightarrow} \Delta \mathcal{I}_{+}
$$
 which gives :

$$
(4) \int_{V_{\pm}} \left[B_{t+\Delta t}^{T} (\mathbf{r}_{t} + \Delta \mathbf{r}) \partial_{t+\Delta t} - B_{t}^{T} \mathbf{r}_{t} \right] dV_{t} = \Delta F
$$

Then, an incremental elastic stress strain law is assumed between the 2nd Piola-Kirchoff stress and the elastic part of the Green strain :

 $\Delta \tau$ = θ , $\Delta \epsilon^2$, and the \sim ببلت The usual finite element technique leads to the following equation, solved in an iterative $max₁$

$$
\sum_{k} P_{\text{max}}(n+1) = \Delta F + \Delta F(n)
$$

where K_t is the stiffness matrix calculated on $\mathfrak{c}_{\mathfrak{k}_{-}}$

 ΔF_{NL} are corrective forces due to the non linear terms.

.n is the iteration index.

Some quasi-Newton acceleration techniques are used to improve the convergence of this algorithm, use clask invitation to material and

4.2⁵ Material non linearity or any

The first approach $\int_0^{\pi} 16 \frac{7}{4} \int_0^{\pi} \cos i \sin i \cos \theta$ treating the cross section as a whole that is to consider if there is or not a plastic hinge at a node. For this purpose, it is necessary to define the vield surface in terms of the generalized stresses of the pipe. These are shown on Figure 4
and include the inner pressure. Of course, the choice of the yield surface is not unique, since
it corresponds to some measure of the state of stresses in an average sens. In TEDEL, a mean of
order 2 has been assumed, which gives :

$$
f(\bar{\sigma}^{\mu}, \bar{\lambda}) \leq 0, \text{ with } z = 0
$$

$$
\sigma^{\mu} = \sqrt{\alpha_{1}^{2} \cdot \frac{N^{\mu} + \alpha_{2}^{2} \cdot N^{\mu}}{N^{\mu} + \alpha_{1}^{2} \cdot N^{\mu} + \alpha_{2}^{2} \cdot N^{\mu} + \alpha_{3}^{2} \cdot N^{\mu}}}
$$

A are internal parameters, and of are coefficients. It must be understood, that with such an approach, the cross section is or is not plastic, while in reality, plasticity spreads continuously over the cross section. However, it is possible to adjust the values of the my coefficients, in order to decide when the plastic hinge is forming. For example, they may be determined on the basis of a limit analysis. Besides, the plastic flow is assumed to be given by the norma-
lity principle, associated with the above criterion. The corresponding strains are generalized strains, like relative length variation and curvatures variations.

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It is possible to account for hardening effects through the A parameters : Isotropic and kinematic hardenings have been implemented as follows :

$$
\sqrt{\sum_{i=1}^{5} \alpha_i^* \cdot \sigma_i^2} \quad \text{if } R (\lambda) \text{ isotropic}
$$
\n
$$
\sqrt{\sum_{i=1}^{5} \alpha_i^* (\sigma_i - x_i)^*} \le R \text{ kinematic}
$$

there the σ_1 are the various generalized stresses (N, My, M₂, C, p) and the x_j are the kinematic
hardening variables. The evolution of these variables is assumed to be of the form:

 $dX_1 = \beta(\sigma_1 - X_1) d\lambda$

which $\frac{1}{2}$ is $\frac{1}{10}$ the classical PRAGER-ZIEGLER type.
A second approach $\sqrt{33}$ $\sqrt{7}$ is $\sqrt{2}$ has been followed, in order to have a better representation
of what happens in the elbows. Indeed, elbows are very flexible if compared whith straight parts and the ovalization of their cross section results in high hoop stresses due to bending mo-
ments. This is accounted for in the global rethod by means of an inertia-modification

 ± 1 ^t $\pm \frac{1}{k}$ where k is the well known flexibility factor. In TEDEL, the ASME formula has been a hataal -

 r = mean radius of the pipe
 R = bend radius

and the yield criterion has been modified accordingly. However, for some applications like calculations of pipes with thermal transients indu-
cing thermal gradients across the thickness, it is desirable to have an accurate description of the state of stresses in the pipe. For this purpose, a special pipe bend has been developped

/ 15 / : it is based on Fourier's series develop-

ments of the local ovalization displacements. Strains are calculated according to the classical Kirchoff-Love thin shell theory.

Assuming a plane state of stresses, stresses are calculated using the classical Prandtl-Reuss
plasticity theory. Then, a condensation technic eliminates the local unknowns in terms of the classical global unknowns. Integrations over the
element volume are performed numerically, using integration points which are regularly distributed over the cross-section (see Figure 5).

Using such elements, it is possible to
follow the spread of plasticity over the crosssection, but it is more costly than the global method.

Therefore, for pipe whip analysis, the glo-
bal method will be preferred.

4.3 - Following force

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It is possible to introduce the force induced by the fluid in two ways :

- either by a nodal force, which is the jet
force and which continuously follows the geometry of the pipe. In TEDEL, it has been treated in an
explicit way, i.e. the following force at time t + At is estimated with the geometry at time t.
This approach is well fitted for small radius pipe bends, since the applied load is locali.ed. Whereas in the case of large radius pipe bends,

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it is advisable to distribute the force induced by the fluid all along the bend, as done below.
- or by an internal dynamical pressure pg

which may yary in the yer long also en way we time:

Pou**p (t) or p(t) TP(t) and t)** sous la ligne brises

where $\phi =$ density

ere y = Gensity
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This pressure may be determined by a preliminary thermohydraulical calculation. In a fully coupled approach, such a calculation will be made together with the mechanical response calculation.

--4.4 - Unilateral contact (18 1/19) ---

The impact of the pipe on a rigid or another the impact of the pipe on a right or another
quality on the nodal displacements as an ine-
quality on the nodal displacements :
 $\frac{1}{2}$ and $\frac{1}{2}$ if $\frac{1}{2}$ is the special state been followed

to tacklewith these inequalities :

- the use of contact elements, which are activated when the inequalities are violated, and which work only in compression. The reaction force developped in the element is of course function of the penetration which is a priori unknown. The solution is found by iterations :
L. $\Delta q^{(n+1)} = \int_0^{\infty} e^{-x} dx + \Delta f^{(n)} = \Delta f^{(n)}$

where L is an operator depending on the wass and
stiffness matrices and the choosen algorithm.

F. depends on the initial conditions at time t

 ΔF is the load increment
 $\Delta F_{\Delta P}^{(m)}$ are the corrective non linear forces

 $\mathbf{F}_{\theta C}^{(n)}$ are the additionnal corrective forces

¹It must be noted that the convergence of such an algorithm is sometimes difficult I Therefore it may be advisable to use the following

The use of Lagrange's multipliers, which leads to a modification of the total notential eneray:

 $-\alpha^2 = 0 + \lambda^2 (A_0 - C)$

The fission forces are found to be 5-175

The usual equilibrium equations supply the
displacements q in terms of the multipliers A. Therefore these may be determined by maximizing a quadratic form under constraints :

 $\frac{1}{\lambda}$ $\frac{1}{\lambda}$

This problem is solved using the FRANK-WOLFE
algorithm \int 18.7, which is proved to converge in
a finite number of steps. $\sin \theta$ was solved

The main advantage of this second approach
is to be fully implicit, if the time integration scheme has been chosen so, and consider the

5. EXAMPLE OF APPLICATION And control

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Pipe Whip tests have been performed by C.E.A. $\sqrt{20.7}$ in France in order to investigate
the behaviour of PWR primary piping under LOCA
conditions. Among them tests of impacts of whipping gipes on rigid targets have been cal-
culated $/$ 21 $/$ 22 $\overline{7}$. The tested pipes were made
of a straight part (430 mm) terminated by a small

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radius bend and a small straight part (see Figure 6).

The dimensions of the pipe were 3" schedule 80 S and the bend radius was 1.5 d.

Two gaps were investigated :

 $h = 180$ nm and $h = 270$ mm.

The material was an austenitic stainless steel (AISI 326 L). Operating pressure and temperature were 16.5 MPa and 320°C.

The guillotine pipe break was initiated by
a pyrotechnical device, with an opening time in-
ferior to one millisecond.

In a first test, a support equipped with load
cells was placed just below the elbow in order to measure the jet force. The result as well as the value introduced in the computation are shown on Figure 7.

During the impact tests, the impact force
measured by a similar device. For the calculations, a simple mesh has been used (see Figure 8).

The office in the spring that been placed under
A non-linear spring this been placed under rigidity of the pipe at impact. The spring stiff-
ness: has been determined by means of a static
compression test on a similar elbow (see Figure 9) The corresponding force-displacement curve is

ine surresponding interesting to the curve is
displayed an interesting the care of the curve is
different gaps: Figure 11 shows the predicted
different gaps: Figure 11 shows the predicted
deformed geometries at impact: The between measured and calculated impact load is
displayed on Figure 19.5% Surrounds.

orsprayed on rigure 19.
It must be noted that, Th these calculations
the influence of the strain rate on the material properties has not been accounted for. Obviously, this should be the case, in particular for the
additional non linear spring. In TEDEL, it is possible to introduce a stress-strain curve depending on the strain-rate; for example, one of
the few laws available in litterature could be
adopted his available in litterature could be
adopted historical depending one of \mathcal{E}_v one.
 \mathcal{E}_v dynamic $(e, k) = \sigma_{\text$

6. CONCLUSION

The TEDEL program has proved to be efficient for calculations of the pipe whip. The global method for plasticity and the Lagrange multipliers technic for unilateral constraints enable rather cheap calculations, with sufficiently large steps. However, the program does not account
for phenomena like the full coupling betwenn the fluid and the pipe, and an accurate description of the impacted zone. This will be treated by the PLEXUS program which is being developped.

7. REFERENCES

- $\sqrt{1}$, $\sqrt{7}$ M. LEPAREUX "Système CEASEMT - Programme PLEXUS" Rapport DEMT/SMTS/BAMS/81-066 (1981)
- $\sqrt{2}$ \sqrt{F} . JEANPIERRE et al. "CEASEMT - System of finite wiement computer programs - Use for inelastic analysis in liquid metal cooled reactor components" IAEA/INGFR Specialists meeting on High Temperature structural Design Technology
Of LMFBRs CHAMPION - Pa (1976)
- /3.7 A. HOFFMANN et al.
"PLEXUS A general computer program for fast dynamic analysis" Proc. of the Conference on Structural

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"Appuis Unilatéraux" Report DENT/SNTS/BANS/80-35 (1980) ℓ 19 ℓ A. MARTIN, A. MILLARD, A. RICARD
The intermittent contact-impact problem In piping systems of nuclear reactors"
Proc. Cong. Num. Meth. for Coupled Pro-
blems - SWANSEA (19 (1981) 2027 P. CAUNETTE, J.L. GARCIA, A. MARTIN
"Experimental studies of PMR primary pi-
----ping under LCCA conditions" Nuclear Engineering and Design, Vol. 61 Anwert 2007-2008 (1980) $\sqrt{21.7 \text{ J.L.}}$ GARCYA, A. MARTIN, P. CHOUARD

"A simplified methodology for calculations

of pipe impacts: Comparison with tests"

ASME paper 82 - MA/PVP2 Tel: J.L. GARCIA, P. CAURETTE, J.L. HUET
Studies of pipe whip and impact
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Fig. 1 - Display of whipping pipe with non linear crush stiffness

 $\mathbf{H}=\mathbf{1}$

 $\mathbf{U} = \mathbf{U} \times \mathbf{U}$.

 $\sim 10^{-10}$ m $^{-1}$

 \mathbb{F}^2 $\sim 1-1$

Fig. 3 - Global behaviour law for reinforced concrete

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Fig. 4 - Generalized stresses for a pipe

Fig. 5 - Integration points over the cross-section

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Fig. S - Model used for computations

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Fig. 9 - Static test for the determination of local crush rigidity of the elbow

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Fig. 10 - Load - Displacement curve

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 $1\leq i\leq 1$

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Fig. 11 - Caculated deformed geometry at impact

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