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**Manuscript for Submission to Nature Climate Change**

**Title: A global framework for future costs and benefits of river flood protection in urban areas**

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18

19 *Introductory paragraph*

20 **Floods cause billions of dollars of damage each year<sup>1</sup>, and flood risks are expected to increase due to**  
 21 **socioeconomic development, subsidence, and climate change<sup>2,3,4</sup>. Implementing additional flood risk**  
 22 **management measures can limit losses, protecting people and livelihoods<sup>5</sup>. Whilst several models**  
 23 **have been developed to assess global-scale river flood risk<sup>2,4,6,7,8</sup>, methods for evaluating flood risk**  
 24 **management investments globally are lacking<sup>9</sup>. We present a framework for assessing costs and**  
 25 **benefits of structural flood protection measures in urban areas around the world. We demonstrate its**  
 26 **use under different assumptions of current and future climate change and socio-economic**  
 27 **development. Under these assumptions, investments in dikes may be economically attractive for**  
 28 **reducing risk in large parts of the world, but not everywhere. In some regions, economically efficient**  
 29 **investments could reduce future flood risk below today’s levels, in spite of climate change and**  
 30 **economic growth. We also demonstrate the sensitivity of the results to different assumptions and**  
 31 **parameters. The framework can be used to identify regions where river flood protection investments**  
 32 **should be prioritised, or where other risk reducing strategies should be emphasised.**

33 *Main text*

34 Recently, a first generation of global river flood risk models has been developed<sup>2,4,6,7,8</sup>. A limitation is  
 35 their assumption that no flood protection infrastructure is in place, leading to overestimations of risk<sup>6</sup>.  
 36 Several studies have assessed flood risk using simple assumptions of current protection standards<sup>3,10,11</sup>.  
 37 However, they did not assess costs and benefits of further investments in increasing flood protection.  
 38 This information is useful for planning investments in flood risk management and adaptation<sup>12,13,14</sup>. Here,  
 39 we demonstrate a framework for cost-benefit analysis of flood risk reduction using the GLOFRIS6<sup>7</sup> global  
 40 flood risk model.

41 First, we used GLOFRIS6<sup>7</sup> to calculate current river flood risk in urban areas, with and without protection.  
 42 Assumptions for current protection standards are from FLOPROS<sup>15</sup> (Supplementary Fig. 1), a database of  
 43 sub-national scale protection standards. Globally, modelled Expected Annual Damage (EAD) is 91%  
 44 lower when estimates of current protection standards are included (\$94 billion versus \$1031 billion).  
 45 Therefore, current protection already provides large societal benefits (Figure 1).

46

47 ***Figure 1 approximately here***

48

49 Since flood protection is not optimal today and risk will change over time, it may be desirable to  
 50 increase protection standards in some regions. We explore three “adaptation objectives” – i.e. three  
 51 approaches to developing risk reduction strategies through dikes. The ‘optimise’ objective prescribes  
 52 protection standards that maximise Net Present Value (NPV). Since optimisation studies are complex  
 53 and rare in practice<sup>13</sup>, we also test two simpler objectives. The ‘constant absolute risk’ objective keeps

54 future EAD constant in absolute terms at current levels, assuming no change in societal preferences  
55 towards absolute risk. The 'constant relative risk' objective keeps future EAD as a percentage of GDP  
56 constant, reflecting a desire to keep flood risk constant as a share of the economy.

57 Aggregated globally, modelled Benefit:Cost (B:C) ratios exceed 1 for all objectives for the scenarios  
58 shown in Table 1. The only exception is RCP6.0/SSP3, for 'constant absolute risk'. Results shown here are  
59 averaged across five global climate models (GCMs), using a 5% per year discount rate and middle-  
60 estimate investment costs (Methods). The four scenarios shown represent plausible combinations of  
61 Representative Concentration Pathways<sup>16</sup> (RCPs) and Shared Socioeconomic Pathways<sup>17</sup> (SSPs). The  
62 scenario selection is described in Supplementary Information 1. Other RCP/SSP combinations are  
63 plausible, so Supplementary Table 1 shows results for all combinations.

64 By definition, highest NPVs are achieved under the 'optimise' objective. Global costs are lowest for  
65 'optimise', and highest for 'constant absolute risk'. For the latter, NPV is on average 61% lower than for  
66 'optimise' (range: 16%-138% lower), whilst for 'constant relative risk', NPV is on average 35% lower than  
67 for 'optimise' (range: 15%-69% lower). Given the high B:C ratios, even if the 'optimise' objective is not  
68 pursued, the simpler objectives are preferable to doing nothing. RCP8.5/SSP5 would entail higher  
69 investments than if more stringent international climate policies achieve lower greenhouse gas  
70 concentrations (Table 1). Sensitivity analysis was carried out using 3% and 8% per year discount rates  
71 (Supplementary Table 2); only for RCP8.5/SSP5 do we see B:C ratios under 1 (for 'constant absolute risk'  
72 and 'constant relative risk' and 8% discount rate).

73

74

75 *Table 1: Globally aggregated results for the ‘optimise’, ‘constant absolute risk’, and ‘constant relative*  
76 *risk’ adaptation objectives. The table shows the average results across the five Global Climate Models,*  
77 *under the following assumptions: middle-estimate investment costs; maintenance costs of 1% per year;*  
78 *and a discount rate of 5% per year. We assumed that the construction of dikes begins in 2020 and is*  
79 *completed by 2050, and that by 2050 dikes are designed to the standard required for the climate at the*  
80 *end of the 21st century (2060-2099). Annual costs are based on the period 2020-2100.*

Adaptation objectives	Scenario			
	RCP2.6/SSP1	RCP4.5/SSP2	RCP6.0/SSP3	RCP8.5/SSP5
<b>Objective: optimise</b>				
Benefits (USD billion per year)	316	254	105	799
Costs (USD billion per year)	47	44	27	78
Benefit:Cost ratio	6.7	5.7	3.9	10.2
NPV (USD billion per year)	269	210	78	721
<b>Objective: constant absolute risk</b>				
Benefits (USD billion per year)	339	276	125	827
Costs (USD billion per year)	170	177	155	219
Benefit:Cost ratio	2.0	1.6	0.8	3.8
NPV (USD billion per year)	169	99	-30	608
<b>Objective: constant relative risk</b>				
Benefits (USD billion per year)	275	225	100	721
Costs (USD billion per year)	73	80	76	108
Benefit:Cost ratio	3.8	2.8	1.3	6.7
NPV (USD billion per year)	202	145	24	613

81

82 Protection standards required per sub-national unit to achieve the ‘optimise’ objective are shown in  
83 Figure 2, with associated B:C ratios in Supplementary Fig. 2. For large parts of North America, Australia,  
84 northern Europe, and East Asia, these optimal standards could decrease future absolute EAD (in 2080)  
85 below current values (Supplementary Fig. 3). However, for most of the world, their implementation  
86 would still lead to overall increases in absolute EAD.

87 Nevertheless, the optimal standards would lead to decreases in future EAD as a percentage of GDP, in  
88 large parts of the world (Supplementary Fig. 4). This is particularly the case for the aforementioned  
89 regions, and for South Asia, Europe, and Central Africa. In the latter regions, flood risk would increase,  
90 but slower than economic growth. Even though our simulations found no protection standards with  
91 positive NPV in many parts of South America, EAD relative to GDP decreases by 2080 in large parts of  
92 southwestern South America. Here, projected economic growth is greater than projected increases in  
93 absolute flood risk.

94

95 **Figure 2 approximately here**

96

97 For individual GCMs, Supplementary Figs. 5-6 show that whilst there are differences between GCMs in  
98 terms of optimal protection standards and B:C ratios, the overall regional patterns are robust in terms of

99 where benefits of additional dikes outweigh costs. This pattern is consistent at 3% and 8% discount rates  
100 (Supplementary Figs. 7-8). For the low-cost estimate (Supplementary Fig. 9), positive NPVs are achieved  
101 for the 'optimise' objective in most regions, including many parts of South America and Africa. For the  
102 high-cost estimate (Supplementary Fig. 10), the general spatial pattern remains, albeit with lower  
103 protection standards and fewer sub-national units where positive NPVs are achieved.

104 B:C ratios per sub-national unit for 'constant absolute risk' and 'constant relative risk' are shown in  
105 Figure 3a and 3b respectively, for RCP4.5/SSP2. Results for the other scenarios can be found in  
106 Supplementary Figs. 11-12 respectively, and corresponding protection standards in Supplementary Figs.  
107 13-14. Whilst future absolute risk could theoretically be contained at today's levels, Figure 3a shows that  
108 doing this with dikes would be economically undesirable in those areas with B:C ratios less than 1.  
109 Generally, B:C ratios are higher for 'constant relative risk' (Figure 3b), although the overall spatial  
110 pattern is similar. Since future hydrological simulations are sensitive to the choice of GCM and scenario<sup>18</sup>,  
111 Figure 3 (c,d) shows the percentage of simulations (over all combinations of five GCMs and four  
112 RCP/SSPs discussed) for which the B:C ratio exceeds 1. Such information is useful, since it identifies  
113 regions with high agreement between models and scenarios, where investments could be prioritised.  
114 The overall spatial pattern is robust using 3% and 8% per year discount rates (Supplementary Figs. 15-  
115 16).

116

117 **Figure 3 approximately here**

118

119 Given the large number of assumptions used, we examine in detail the sensitivity of the results to  
120 different: RCP/SSPs; GCMs; discount rates; cost estimates; and baseline protection standards. Results for  
121 all assumptions are described in Supplementary Information 2 and made available in the Supplementary  
122 Dataset. An important uncertainty stems from the estimates of current flood protection standards from  
123 FLOPROS (Methods). The framework allows this standard to be changed, when better information is  
124 available from users. We test the sensitivity of results to this assumption by also carrying out simulations  
125 assuming current flood protection to be: (a) half that in FLOPROS (Supplementary Figs. 17-18 &  
126 Supplementary Table 3); and (b) double that in FLOPROS (Supplementary Figs. 19-20 & Supplementary  
127 Table 4). The results are robust in terms of their influence on the B:C ratio and the order of magnitude of  
128 the optimal flood protection standard.

129 We also assess the robustness of the results to the various assumptions. In Figure 4, we show protection  
130 standards and B:C ratios for two simulations - under the 'optimise' objective - at either end of the  
131 parameter spectrum. We selected RCP/SSP combinations providing the highest (RCP8.5/SSP5) and  
132 lowest (RCP2.6/SSP3) B:C ratios. We combined RCP8.5/SSP5 with low-cost estimates and a 3% per year  
133 discount rate (a,c). We combined RCP2.6/SSP3 with high-cost estimates and an 8% per year discount  
134 rate (b,d). Globally, these represent the simulations with the highest and lowest benefits relative to  
135 costs, respectively. Whilst values for benefits and costs are different, Figure 4 shows the overall spatial  
136 patterns in B:C ratios and protection standards to be consistent. In Supplementary Information 2.6 we

137 discuss overall robustness; Supplementary Figure 21 shows the percentage of simulations in which the  
138 B:C ratio exceeds 1 across all 2700 combinations of parameters discussed in this letter, showing the  
139 conclusions to be very robust in many regions.

140

141 **Figure 4 approximately here**

142

143 In this paper, we used the GLOFRIS inundation model, which uses a volume-spreading algorithm, rather  
144 than a hydrodynamic scheme. More complex hydrodynamic models can potentially simulate present-  
145 day inundation more accurately<sup>19</sup>. However, of the six models used in a recent comparison study of  
146 global flood models<sup>20</sup>, GLOFRIS is the only one that has been used to simulate high-resolution  
147 inundation under future climate scenarios. For two of the other models, the use of regional flood  
148 frequency curves instead of climate input data means that future simulations cannot be performed in  
149 the current setup. For the other models, long runtimes have meant that any future simulations have  
150 only been carried out at lower resolution, or on the discharge component only. We tested whether  
151 GLOFRIS is able to simulate inundation with high enough skill in urban areas so that the flood impact  
152 results do not deviate excessively from impact results based on more accurate inundation maps. To do  
153 this, we carried out the most extensive benchmarking experiment to date of global model results  
154 compared to local data (Supplementary Information 3). For eight case studies, we compared GLOFRIS  
155 inundation maps with inundation maps from local models or satellite imagery. We find that GLOFRIS  
156 simulates inundation extent in urban areas as well as it simulates inundation extent elsewhere. We also  
157 used both the GLOFRIS and benchmark inundation maps to simulate flood impacts (potential maximum  
158 damage). We find that the percentage differences in maximum potential damage using the GLOFRIS and  
159 benchmark inundation maps is much lower than the differences in EAD caused by the use of different  
160 flood protection standards, and the use of different GCMs, RCPs, and SSPs. We have already shown the  
161 overall conclusions to be robust to the latter assumptions.

162 In future studies, it would be useful to carry out full uncertainty assessments across multiple models  
163 such as the multi-modelling exercises carried out for the Inter-Sectoral Impact Model Intercomparison  
164 Project (ISIMIP). The value of multi-model studies has been shown at European scale using high-  
165 resolution ensembles from Regional Climate Models<sup>21</sup>. As higher resolution global climate data become  
166 available, and the number of global flood inundation models increases, our framework could be used to  
167 provide multi-model assessments of adaptation benefits and costs. Indeed, the framework can be used  
168 with inputs from different models, and model parameters can be adjusted based on local knowledge  
169 from users.

170 Increased structural flood protection appears economically attractive in large parts of the world. In  
171 some regions, implementing protection standards that maximise-NPV can negate absolute increases in  
172 risk that would otherwise occur. However, we also show where structural protection can be  
173 economically unviable. In those regions, more attention is required for other flood risk management  
174 strategies<sup>5</sup>. In practice, feasibility of flood protection is not only related to the economic parameters

175 used here. Other factors that may render structural defences infeasible include: the presence of humans  
176 and livelihood activities, soil subsidence, or reduced sediment accretion; or non-economic factors  
177 related to structural defences such as loss of existing amenities, tourism, ecosystems, and fisheries. Also,  
178 the construction of structural flood protection measures can lead to lock-in and the so-called levee  
179 effect<sup>22</sup>. Optimal risk reduction strategies are therefore usually a mix of different measures<sup>12</sup>. Future  
180 studies should consider costs and benefits of multiple adaptation strategies, such as retention areas,  
181 flood-proofing buildings, early warning systems<sup>23</sup>, building codes<sup>12</sup>, and post-disaster support. Whilst we  
182 limit our analyses to built infrastructure, green measures can also reduce flood risk<sup>24</sup>. In some regions,  
183 green measures already provide some flood protection. Information on multiple strategies is essential  
184 for integrating disaster risk management into broader development policy discussions, in which trade-  
185 offs must be made between risk reduction and other issues, like health and education. This is  
186 particularly the case in low-income countries, where financial resources struggle to satisfy needs<sup>25</sup>. On  
187 the other hand, in many regions floods disproportionately affect poor people<sup>25</sup>, so risk reduction is  
188 commensurate with overall development goals. The current study only considers direct economic  
189 damages, whilst floods can also cause extensive indirect damages<sup>26</sup>, fatalities, and injury<sup>27</sup>. Methods are  
190 required to integrate these into global scale risk assessments, since these impacts also influence flood  
191 protection effectiveness.

192 Our framework can be used to highlight potential savings through strategies to increase structural flood  
193 protection at the sub-national scale. When moving towards implementation of individual measures,  
194 detailed studies should be performed using local models and data<sup>9,28</sup>, but global analyses help to initiate  
195 dialogue with stakeholders and identify priority regions. To increase accessibility to the risk community,  
196 results of this study will be integrated into the Aqueduct Global Flood Analyzer webtool  
197 ([www.wri.org/floods](http://www.wri.org/floods)).

198

#### 199 **Statement on corresponding author**

200 Correspondence and requests for materials should be addressed to Philip J. Ward.

201

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210



211 **Author contributions**

212 All authors conceived and designed the experiments and contributed to discussions on, and writing of,  
213 the paper. P.J.W., B.J., P.S., and H.C.W. performed the experiments. P.J.W., A.D.L., P.S., and H.C.W.  
214 analysed the data. P.D.B. contributed to the benchmarking exercise.

215

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274

275 **Figure captions**

276 Figure 1: Percentage reduction in current expected annual damage for simulations carried out with  
277 assumed current protection standards compared to no flood protection.

278 Figure 2: Protection standards at sub-national level in 2080 that meet the 'optimise' objective, for: (a)  
279 RCP2.6/SSP1; (b) RCP4.5/SSP2; (c) RCP6.0/SSP3; and (d) RCP8.5/SSP5. The average return period is  
280 shown across the five GCMs. Sub-national units in which no increase in protection standard provides a  
281 positive NPV are indicated by N/A. Results are shown assuming middle-estimate investment costs,  
282 maintenance costs of 1% per year, and a discount rate of 5% per year.

283 Figure 3: Panels (a) and (b) show B:C ratio at sub-national level for the following adaptation objectives:  
284 (a) EAD-constant; and (b) EAD/GDP-constant. Panels (c) and (d) shows how often the B:C ratio exceeds 1,  
285 as a percentage of the simulations for all five GCMs and the following four RCP/SSP combinations:  
286 RCP2.6/SSP1; RCP4.5/SSP2; RCP6.0/SSP3; and RCP8.5/SSP5 (i.e.  $n = 20$ ). Results are shown here  
287 assuming middle-estimate investment costs, maintenance costs of 1% per year, and discount rate of 5%  
288 per year.

289 Figure 4: Panels (a) and (b) show protection standards at sub-national level in 2080 that meet the  
290 'optimise' objective; and (c) and (d) show the B:C ratios associated with (a) and (b) respectively. The  
291 simulations are used to show the robustness of the results (in terms of simulated protection standards  
292 and B:C ratios) across the different combinations of RCPs/SSPs, estimates of investment costs, and  
293 discount rates. (a) and (c) are based on the low-estimates of investments costs, a discount rate of 3%  
294 per year, and RCP8.5/SSP5. This combination represents the highest B:C ratio at the globally aggregated  
295 scale. (b) and (d) are based on the high-estimates of investments costs, a discount rate of 8% per year,  
296 and RCP2.6/SSP3. This combination represents the lowest B:C ratio at the globally aggregated scale.

297

## 298 **Methods**

299 We developed a method to assess the benefits and costs of reducing future river flood risk (expressed as  
300 expected annual damage, EAD, in urban areas) at the sub-national scale, by increasing flood protection  
301 standards offered by dikes. Here, urban refers to all kinds of built-up areas and artificial surfaces. Sub-  
302 national scale is defined as the next administrative unit below national scale in the Global Administrative  
303 Areas Database (GADM). We used the method to assess the benefits and costs of three adaptation  
304 objectives: (a) maximising NPV of the investment (optimise); (b) keeping future EAD constant in absolute  
305 terms (constant absolute risk); and (c) keeping future EAD as a percentage of GDP constant (constant  
306 relative risk). In brief, benefits of increasing structural protection are defined as the difference between:  
307 future EAD if dikes remain constant at assumed current height and future EAD if the height of dikes is  
308 increased. Since we do not have global projections of subsidence, this factor is not included. Costs are  
309 defined as the sum of investment and capitalised maintenance costs. The different steps are described  
310 in the following paragraphs.

### 311 *Calculation of EAD*

312 Urban damage was calculated at sub-national scale using GLOFRIS<sup>6,7</sup> for several return periods (2, 5, 10,  
313 25, 50, 100, 250, 500 and 1000 years), and EAD was calculated as the integral of the area under an  
314 exceedance probability-damage curve (risk curve) across these different return periods. Validation of  
315 GLOFRIS in past studies, and further benchmarking for this study, are described in Supplementary  
316 Information 3. To account for flood protection standards, the risk curve was truncated for return periods  
317 lower than or equal to the protection standard. For example, if a sub-national unit is assumed to have a  
318 flood protection standard of 25 years, damages associated with floods up to and including that return  
319 period were set to zero prior to integration. For return periods exceeding the protection standard, it is  
320 assumed that flood protection does not affect the flood extent. For each future simulation (i.e. each  
321 combination of 1 GCM, 1 RCP, and 1 SSP), EAD was calculated under future (2080) and current (1960)  
322 conditions, and the factor difference between these was calculated. This factor was then applied to the  
323 EAD estimate based on the current data to estimate bias-corrected future EAD. Current protection  
324 standards were taken from the modelled layer of the FLOPROS dataset<sup>15</sup>. FLOPROS provides modelled  
325 protection standards at the sub-national scale. These modelled protection standards have been  
326 validated against actual flood protection standards in place in several regions in Ref. 15. Sensitivity to  
327 this assumption is assessed by re-running the analyses assuming: current flood protection to be: (a) half  
328 that stated in the FLOPROS database; and (b) double that stated in the FLOPROS database  
329 (Supplementary Information 2.5). Since flood inundation is not simulated hydrodynamically, the  
330 framework does not account for the transfer of risk from better-protected upstream areas to  
331 downstream areas.

332 To calculate the urban damage for the individual return periods, we used the GLOFRIS model<sup>6,7</sup>. The  
333 GLOFRIS setup and input data used to carry out the damage simulations used for the current and future  
334 periods in this study are described in detail in Ref.3. We refer the reader to these papers for details of  
335 this model and the setup used in the current paper; here, we provide a brief overview for the sake of  
336 clarity. In essence, the cascade involves: (a) hydrological and hydraulic modelling to develop daily time-

337 series of flood volumes; (b) extreme value statistics to estimate flood volumes for different return  
338 periods; (c) inundation modelling for different return periods; and (d) impact modelling.

339 (a) Hydrological and hydraulic modelling to develop daily time-series of flood volumes: Daily gridded  
340 discharge and flood volumes were simulated ( $0.5^\circ \times 0.5^\circ$ ) using PCR-GLOBWB-DynRout<sup>29</sup>, which requires  
341 daily gridded meteorological input data (precipitation, temperature, global radiation). Validation is  
342 described in Refs.6,29,30. For current climate conditions, EU-WATCH forcing data<sup>31</sup> were used for the  
343 period 1960-1999. For future climate conditions, the forcing data were daily bias-corrected outputs<sup>32</sup>  
344 from the following Global Climate Models (GCMs): HadGEM2-ES, IPSL-CM5A-LR, MIROC-ESM-CHEM,  
345 GFDL-ESM2M, and NorESM1-M. For each GCM, daily gridded discharge and flood volumes were  
346 simulated for the (model) periods 1960-1999 and 2060-2099 (represent climate conditions in 2080).

347 (b) Extreme value statistics: From each of the daily gridded flood volume time-series, annual  
348 hydrological year time-series of maximum flood volumes were extracted, using the approach described  
349 in Ref. 6. Then, we fit a Gumbel distribution through these time-series, based on non-zero data, and  
350 used the resulting Gumbel parameters per grid-cell to estimate flood volumes for the following return  
351 periods (2, 5, 10, 25, 50, 100, 250, 500, and 1000 years), conditioned to years in which zero flood  
352 volumes were exceeded. This produces coarse resolution ( $0.5^\circ \times 0.5^\circ$ ) maps of flood volume for each  
353 return period.

354 (c) Inundation modelling: The coarse resolution flood volume maps were then converted into high  
355 resolution (30" x 30") inundation maps using the inundation downscaling module of GLOFRIS<sup>7</sup>. This  
356 model was used because it is the only global flood hazard model of those assessed in a recent  
357 comparison study<sup>20</sup> that simulates high-resolution inundation under future climate scenarios. The  
358 models compared in that study are: CaMa-Flood<sup>33</sup>, JRC<sup>34</sup>, ECMWF<sup>35</sup>, SSBN-Bristol<sup>19</sup>, CIMA-UNEP<sup>8</sup>, and  
359 GLOFRIS. The CaMa-Flood model has been used to examine changes in flood risk for several future  
360 scenarios<sup>2</sup>. However, due to the large computational time requirement, they only simulated the fraction  
361 of inundation per 2.5' x 2.5' cell, a much lower resolution than GLOFRIS, and not including flood depths.  
362 The JRC model has been used to assess future changes in flood risk<sup>36</sup>. However, they only used  
363 inundation maps at 30" x 30" (i.e. the same as GLOFRIS) based on current climate. They then used a low  
364 resolution global hydrological model ( $0.5^\circ \times 0.5^\circ$ ) to simulate changes in discharge in the future. They did  
365 not simulate future inundation, but used changes in future discharge to adjust the probability of  
366 flooding in the future. This approach was specifically chosen to "*optimize the trade-off between*  
367 *information content and computing resources needed*". To the best of our knowledge, the ECMWF  
368 model has not been used to assess inundation for future scenarios. The SSBN-Bristol and CIMA-UNEP  
369 models use information from regional flood frequency analysis to derive flood hydrographs. They are  
370 not directly forced by climate input data, and therefore their current setup does not allow for future  
371 climate change studies.

372 GLOFRIS employs a volume spreading algorithm, rather than a hydrodynamic modelling scheme. Whilst  
373 it may be preferable to use more complex hydrodynamic models if the aim of the study is to simulate  
374 present-day inundation as accurately as possible, when carrying out a scenario modelling exercise such  
375 as the one carried out for this letter, an important consideration is whether the model provides

376 reasonable performance but also produces inundation maps within a reasonable time-frame and for an  
377 acceptable computational cost. In the case of this study, the important consideration is whether the  
378 model can simulate inundation with high enough skill so that the flood impact results do not deviate  
379 excessively from impact results based on a higher resolution benchmark dataset. We have tested this  
380 extensively, as discussed in Supplementary Information 3.2.3.

381 (d) Impact modelling: Each high resolution inundation map was combined with gridded socioeconomic  
382 data, also at a horizontal resolution of 30" x 30", to calculate urban damage per grid-cell, and these data  
383 were then aggregated to the sub-national scale. In GLOFRIS, urban damage is calculated using the  
384 inundation maps to represent hazard, a map of asset values in urban areas to represent exposure, and a  
385 depth-damage function to represent vulnerability<sup>6</sup>. The asset value map is based on a percentage urban  
386 area per grid-cell multiplied by an estimate of urban asset values per square kilometre. Data for current  
387 urban area per grid-cell were taken from the HYDE database<sup>37</sup>, and data for current urban asset values  
388 were taken from Ref. 6. In the HYDE dataset, and therefore in this study, urban refers to all kinds of  
389 built-up areas and artificial surfaces. Future changes in urban densities and asset values were taken from  
390 Ref. 3, and were computed using gridded population and GDP data from the GISMO/IMAGE model<sup>38,39</sup>,  
391 using the method described in Ref. 40. For the future scenarios of GDP and population, data were used  
392 from the Shared Socioeconomic Pathway (SSP) database<sup>41</sup>. The spatial resolution of the exposure data is  
393 the same as the spatial resolution of the hazard data, so they are commensurate for impact assessment.  
394 Validation of the urban damage values has been carried out for several countries in past studies<sup>6,42</sup>.

#### 395 *Estimation of benefits*

396 Benefits were calculated as the difference between future EAD with and without additional flood  
397 management investments. First, we estimated EAD assuming that no additional investment takes place  
398 in the future compared to current. Effectively, this means that existing dikes are maintained at their  
399 current height. We then estimated, per sub-national unit, the protection standard required in 2080  
400 (under different combinations of RCP/SSP) to achieve the 'optimise', 'constant absolute risk', and  
401 'constant relative risk' objectives. The maximum protection standard is capped at 1000 years, since this  
402 is the largest return period for which damages are physically simulated in GLOFRIS.

#### 403 *Estimation of costs*

404 Costs are calculated by summing investment and capitalised maintenance costs. All costs reported in  
405 this letter are in USD2005 at Purchasing Power Parity (PPP), and were adjusted from the original values  
406 stated in the literature using GDP deflators from the World Bank, and annual average market exchange  
407 rates between Euros and USD taken from the European Central Bank. The cost estimates described  
408 below are in constant USD, and are adjusted to PPP values in the model (using World Bank converters),  
409 since the benefits derived from GLOFRIS are also in PPP values.

410 First, we estimate the investment costs of dikes in the USA. Cost estimates in the literature vary widely,  
411 as shown in several recent overview papers<sup>43,44,45</sup>. To account for this variation, here we applied three  
412 cost estimates: high, middle, and low. For the middle-cost estimate, we use a value of USD 7.0 million  
413 km/m heightening. This estimate is based on reported costs in New Orleans in Ref. 46. It pertains to all

414 investments costs, including ground work, construction, and engineering costs, property or land  
 415 acquisition, environmental compensation, and project management. We selected this value since it also  
 416 is in the middle of other recent estimates in Refs. 43 and 44 from the USA and the Netherlands.  
 417 Moreover, it is close to the average cost of heightening reported in Ref. 47 of USD 6.6 million km/m  
 418 heightening, for 21 dike-rings in the Netherlands; USD 6.7 million km/m heightening for 36 dike-reaches  
 419 in Canada reported in Ref. 45; and USD 8.4 million km/m heightening for coastal dikes in the  
 420 Netherlands reported in Ref. 45. In a recent study based on empirical investment cost data from the  
 421 Netherlands and Canada, Ref. 45 found that investment costs per metre heightening are well described  
 422 by a linear function without intercept. They conclude that for large scale studies it is sufficient to assume  
 423 linear costs for each metre of heightening, including the initial costs, and therefore we assumed this to  
 424 be the case for the current study. These cost estimates were then adjusted for all other countries by  
 425 applying construction index multipliers<sup>48</sup> (based on civil engineering construction costs) to account for  
 426 differences in construction costs across countries<sup>49</sup>. The empirical investigation of dike costs in Ref. 45  
 427 also found that the spread in cost estimates caused by factors other than dike length and height can be  
 428 well represented by assuming low and high costs estimates of 3x and x/3, where x represents the best  
 429 cost-estimate. Therefore, we also used this approach to carry out our benefit:cost analyses for a low-  
 430 cost estimate (USD 2.3 million km/m heightening) and for a high-cost estimate (USD 21.0 million km/m  
 431 heightening). We assumed maintenance costs to be 1% per year of investment costs<sup>44</sup>.

432 We estimated the kilometre length of dikes required by combining the river network map and the map  
 433 of urban areas used in GLOFRIS (both 30" x 30"). We calculated the length of rivers of Strahler order 6 or  
 434 higher (since these are the rivers for which inundation is simulated in GLOFRIS) flowing through urban  
 435 areas, i.e. areas that are indicated as urban in the HYDE database.

436 To calculate the costs of dike heightening, an estimate is also required of the (increase in) dike height  
 437 needed for each future scenario to facilitate protection against floods for various magnitudes and  
 438 associated return periods. For each 0.5° x 0.5° grid cell, we estimated the required height of the dike for  
 439 a given return period of protection by converting the discharge occurring with the return period into a  
 440 flow depth. For a given scenario and protection level, and for a given grid cell, we established the  
 441 heights of the dikes as follows. First we retrieve the discharge occurring with the return period  
 442 associated with the required protection level from a Gumbel distribution of discharges, established from  
 443 GLOFRIS as described in Ref. 6. Dikes are usually not built directly at the banks of the river, but at a  
 444 certain distance from the banks within the floodplain. We have here assumed that they are built at a  
 445 distance of one times the channel width from the river banks. The width and bankfull depth of the  
 446 channel are taken from the hydrological model PCRGLOB-WB (part of GLOFRIS framework), using:

$$447 \quad Q = hB \frac{1}{n} R^{2/3} \sqrt{i} \quad (\text{Eq. 1})$$

448 where  $Q$  is the discharge [ $L^3 T^{-1}$ ],  $h$  is the flow depth [L],  $B$  is the flow width [L],  $n$  is the Manning  
 449 roughness [ $T L^{-1/3}$ ],  $R$  is the hydraulic radius [L] (equal to  $hB/(2h+B)$ ) and  $i$  is the slope of the channel [-].  
 450 In large rivers, flow depth is much smaller than the flow width, and  $R$  can be approximated by  $h$ ,  
 451 reducing Eq. 1 into:

452  $Q = B \frac{1}{n} h^{5/3} \sqrt{i}$  (Eq. 2)

453 In our case, a part of the flow is through the main channel and part over the part of the floodplain that  
 454 lies in between the dikes, both having different dimensions and roughness values. We therefore split up  
 455 Eq. 2 into a channel part and a floodplain part as follows:

456  $Q = \left[ B_c \frac{1}{n_c} h^{5/3} + B_f \frac{1}{n_f} (h - h_{bf})^{5/3} \right] \sqrt{i}$  (Eq. 3)

457 where  $c$  and  $f$  are channel and floodplain respectively, and  $h_{bf}$  is the bankfull channel depth [ $L$ ]. We solve  
 458 this equation for  $h$ . The required height of the dike is then  $h-h_{bf}$ .

459

#### 460 *Cost-benefit analysis*

461 To carry out the cost-benefit analysis, several assumptions are required. Firstly, the discount rate; we  
 462 used a real discount rate of 5% per year, and performed sensitivity analysis using 3% and 8% per year  
 463 Secondly, we assumed the protection level increases linearly between 2020 and 2050, and that by 2050  
 464 dikes are designed to the standard required for the climate at the end of the 21<sup>st</sup> century (2060-2099).  
 465 The flows of costs and benefits are discounted until 2100.

466

#### 467 *Data availability*

468 The data that support the findings of this study are available from the corresponding author upon  
 469 request. The costs and benefits per sub-national unit are available within the article [and its  
 470 supplementary information files] for all RCP/SSP combinations; each individual GCM; different discount  
 471 rates; high, middle, and low cost estimates; and different assumptions on assumed baseline protection.

472

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